

Step 11 :- Final Moment .

$$\begin{bmatrix} M_{AB} \\ M_{BA} \\ M_{BC} \\ M_{CB} \end{bmatrix} = \begin{bmatrix} -7.47 \\ 3.7 \\ -3.7 \\ 0 \end{bmatrix}$$

Step 12 :- Maximum Bending Moment .

$$M_{AB} = \frac{wl^2}{8} = \frac{3 \times 5^2}{8} = 9.37 \text{ kNm}$$

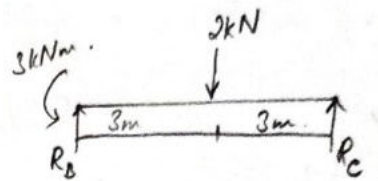
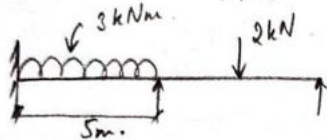
$$M_{BC} = \frac{wl}{4} = \frac{2 \times 6}{4} = 3 \text{ kNm}$$

Step 13 :- Shear Force .

$$\Sigma V = 0$$

$$R_A + R_B + R_C = (3 \times 2) + 5$$

$$= 17 \text{ kN}$$



Span BC :-

$$R_C \times 6 = (2 \times 3) - 3.7$$

$$R_C = 0.38 \text{ kN}$$

Span ABC :-

$$R_C \times 11 + (R_B \times 5) = (2 \times 8) + (3 \times 5 \times 5/2)$$

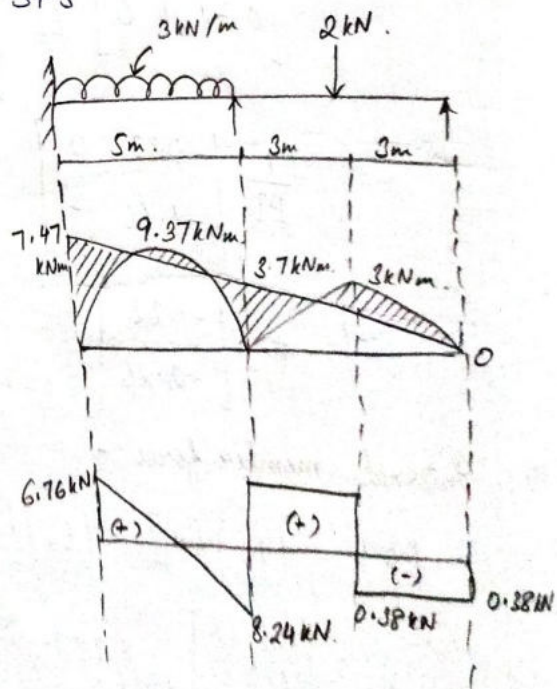
$$(11 \times 0.38) + (R_B \times 5) = 16 + 37.5$$

$$R_B = 9.86 \text{ kN}$$

$$R_A + R_B + R_C = 17$$

$$R_A + 9.86 + 0.38 = 17$$

$$R_A = 6.76 \text{ kN}$$



Date:- 5/04/2022

Q3. Analyze the frame as shown in fig. by flexibility matrix method :-

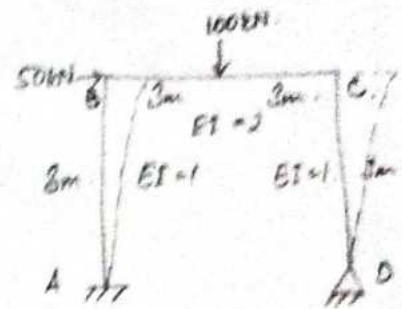
Step 1:- Static Indeterminacy.

$$D_s = R - K$$

$$= 5 - 3$$

$$= 2$$

Support D is selected as an redundant.
 Das redundant
 Gas it has two forces.



Step 2:- Fixed End Moments.

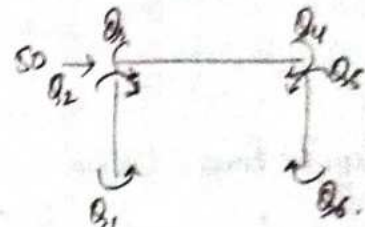
$$M_{FAB} = M_{FBA} = M_{FCD} = M_{FDC} = 0$$

$$M_{FBC} = -\frac{wl}{8} = -\frac{100 \times 6}{8} = -75 \text{ kNm}$$

$$M_{FCB} = \frac{wl}{8} = \frac{100 \times 6}{8} = 75 \text{ kNm}$$

Step 3:- Internal member force action :-

$$Q = \begin{Bmatrix} 50 \\ 75 \\ -75 \end{Bmatrix}$$

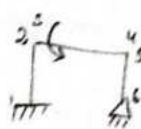


Step 4:- Equivalent joint moment.

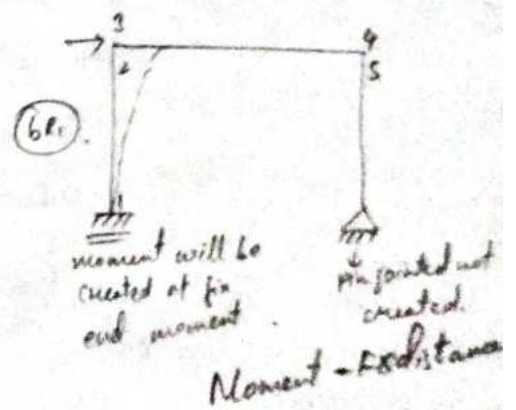
$$R = \begin{Bmatrix} 50 \\ 75 \\ -75 \end{Bmatrix}$$

Step 5:- Force transformation matrix.

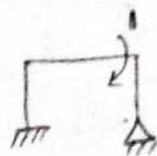
$$\{bR_1\} = \begin{Bmatrix} 3 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \end{Bmatrix}$$



$$\{bR_2\} = \begin{Bmatrix} 0 \\ 0 \\ 1 \\ 0 \\ 0 \\ 0 \end{Bmatrix}$$



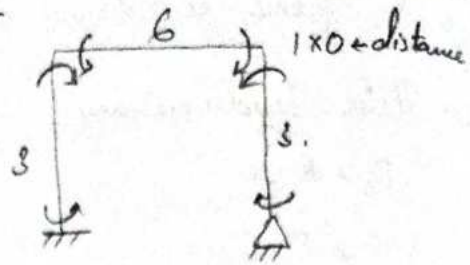
$$\{bR_3\} = \begin{Bmatrix} 0 \\ 0 \\ 0 \\ 1 \\ 0 \\ 0 \end{Bmatrix}$$



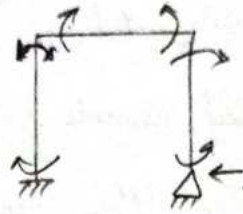
$$\{bR\} = \begin{bmatrix} 3 & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \\ 0 & 0 & 0 \\ 0 & 0 & 0 \end{bmatrix}$$

Step 6:- Moment transformation matrix:-

$$bx_1 = \begin{Bmatrix} -6 \\ 6 \\ 6 \\ 0 \\ 0 \\ 0 \end{Bmatrix}$$



$$bx_2 = \begin{Bmatrix} 0 \\ -3 \\ 3 \\ -3 \\ 3 \\ 0 \end{Bmatrix}$$



$$bx = \begin{Bmatrix} -6 & 0 \\ 6 & -3 \\ -6 & 3 \\ 0 & -3 \\ 0 & 3 \\ 0 & 0 \end{Bmatrix}$$

Step 7:- Force Matrix.

$$F_{AB} = \frac{l}{6EI} \begin{bmatrix} 2 & -1 \\ -1 & 2 \end{bmatrix}$$

$$= \frac{3}{6EI} \begin{bmatrix} 2 & -1 \\ -1 & 2 \end{bmatrix}$$

$$= \frac{3}{6 \times 1} \begin{bmatrix} 2 & -1 \\ -1 & 2 \end{bmatrix}$$

$$= \begin{bmatrix} 1 & -0.5 \\ -0.5 & 1 \end{bmatrix}$$

$$F_{BC} = \frac{l}{6EI} \begin{bmatrix} 2 & -1 \\ -1 & 2 \end{bmatrix}$$

$$= \frac{6}{6 \times 2} \begin{bmatrix} 2 & -1 \\ -1 & 2 \end{bmatrix}$$

$$= \begin{bmatrix} 1 & -0.5 \\ 0.5 & 1 \end{bmatrix}$$

$$F_{CD} = \frac{3}{6 \times 1} \begin{bmatrix} 2 & -1 \\ -1 & 2 \end{bmatrix} = \begin{bmatrix} 1 & -0.5 \\ -0.5 & 1 \end{bmatrix}$$

$$F = \begin{bmatrix} 1 & -0.5 & 0 & 0 & 0 & 0 \\ -0.5 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & -0.5 & 0 & 0 \\ 0 & 0 & -0.5 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & -0.5 \\ 0 & 0 & 0 & 0 & -0.5 & 1 \end{bmatrix}$$

Step 8:- Flexibility Matrix.

$$F_{xx} = [b_x]^T [F] [b_x]$$

$$= \begin{bmatrix} -6 & 6 & -6 & 0 & 0 & 0 \\ 0 & -3 & 3 & -3 & 3 & 0 \end{bmatrix} \begin{bmatrix} 1 & -0.5 & 0 & 0 & 0 & 0 \\ -0.5 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & -0.5 & 0 & 0 \\ 0 & 0 & -0.5 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & -0.5 \\ 0 & 0 & 0 & 0 & -0.5 & 1 \end{bmatrix} \begin{bmatrix} -6 & 0 \\ 6 & -3 \\ -6 & 3 \\ 0 & -3 \\ 0 & 3 \\ 0 & 0 \end{bmatrix}$$

(6x6) (6x2)

$$= \begin{bmatrix} -6 & 6 & -6 & 0 & 0 & 0 \\ 0 & -3 & 3 & -3 & 3 & 0 \end{bmatrix} \begin{bmatrix} -6-3 & 4.5 \\ 3+6 & -3 \\ -6 & 3+1.5 \\ 0 & -3-2.5 \\ 0 & -1.5 \\ 0 & 0 \end{bmatrix}$$

(2x6)

$$= \begin{bmatrix} -6 & 6 & -6 & 0 & 0 & 0 \\ 0 & -3 & 3 & -3 & 3 & 0 \end{bmatrix} \begin{bmatrix} -9 & 1.5 \\ 9 & -3 \\ -6 & 4.5 \\ 0 & -4.5 \\ 0 & -1.5 \\ 0 & 0 \end{bmatrix} \begin{bmatrix} -9 & 1.5 \\ 9 & -3 \\ -6 & 4.5 \\ 3 & -4.5 \\ 0 & 3 \\ 0 & -1.5 \end{bmatrix}$$

$$= \begin{bmatrix} +54 + 54 + 36 & -9 - 18 - 27 \\ -27 + 18 & 9 + 13.5 + 9 + 13.5 \end{bmatrix}$$

$$= \begin{bmatrix} 144 & -54 \\ -54 & 45 \end{bmatrix}$$

→

$$= \begin{bmatrix} 54 + 54 + 36 & -9 - 18 - 27 \\ -27 - 18 - 9 & 9 + 13.5 + 9 + 13.5 \end{bmatrix}$$

Step 9:-

$$[F_{xx}] = \begin{bmatrix} 144 & -54 \\ -54 & 45 \end{bmatrix}$$

$$[A^{-1}] = \frac{1}{|A|} (\text{adj } A)$$

$$[F_{xx}]^{-1} = \frac{1}{8564} \begin{bmatrix} 45 & 54 \\ 54 & 144 \end{bmatrix} = \begin{bmatrix} 0.012 & 0.015 \\ 0.015 & 0.040 \end{bmatrix}$$

Step 9 :- Flexibility matrix).

$$[F_{xx}] = (b_x)^T (F) (b_x)$$

$$= \begin{bmatrix} -6 & 6 & -6 & 0 & 0 & 0 \\ 0 & -3 & 3 & -3 & 3 & 0 \end{bmatrix} \begin{bmatrix} 1 & -0.5 & 0 & 0 & 0 & 0 \\ 0.5 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & -0.5 & 0 & 0 \\ 0 & 0 & -0.5 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & -0.5 \\ 0 & 0 & 0 & 0 & -0.5 & 1 \end{bmatrix} \quad (6 \times 6)$$

$$= \begin{bmatrix} -6 & 6 & -6 & 0 & 0 & 0 \\ 0 & -3 & 3 & -3 & 3 & 0 \end{bmatrix} \begin{bmatrix} 3 & 0 & 0 \\ -1.5 & 0 & 0 \\ 0 & 1 & -0.5 \\ 0 & -0.5 & 1 \\ 0 & 0 & 0 \\ 0 & 0 & 0 \end{bmatrix} \begin{bmatrix} 3 & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \\ 0 & 0 & 0 \\ 0 & 0 & 0 \end{bmatrix} \quad (2 \times 6) \quad (6 \times 3) \quad (6 \times 6)$$

$$= \begin{bmatrix} -18 & -18 & 0 \\ 0 & 9+1.5 & -3 \end{bmatrix}$$

$$= \begin{bmatrix} -18 & -18 & 0 \\ 0 & 10.5 & -3 \end{bmatrix} = \begin{bmatrix} -18-9 & -6 & 3 \\ 18.5 & 3+1.5 & -1.5-3 \end{bmatrix}$$

$$= \begin{bmatrix} -27 & -6 & 3 \\ 4.5 & 4.5 & -4.5 \end{bmatrix}$$

Step 10:- Internal member force.

$$[Q] = [b_x] [R] + [b_x] [x] + FEM$$

$$= \begin{bmatrix} 3 & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \\ 0 & 0 & 0 \\ 0 & 0 & 0 \end{bmatrix} \begin{bmatrix} 50 \\ 75 \\ -75 \end{bmatrix} + \begin{bmatrix} -6 & 0 \\ 6 & -3 \\ -6 & 3 \\ 0 & -3 \\ 0 & 3 \\ 0 & 0 \end{bmatrix} \begin{bmatrix} 10.8 \\ -5.63 \end{bmatrix} + \begin{bmatrix} 0 \\ 0 \\ -75 \\ 75 \\ 0 \\ 0 \end{bmatrix}$$

$$[x] = - [F_{xx}]^{-1} [F_{xR}] [R]$$

$$= - \begin{bmatrix} 0.012 & 0.015 \\ 0.015 & 0.040 \end{bmatrix} \begin{bmatrix} -27 & -6 & 3 \\ 45 & 45 & -45 \end{bmatrix} \begin{bmatrix} 50 \\ 75 \\ -75 \end{bmatrix}$$

(2x3) (3x1)

$$= - \begin{bmatrix} 0.012 & 0.015 \\ 0.015 & 0.040 \end{bmatrix} \begin{bmatrix} -1350 - 450 - 225 \\ 225 + 337.5 + 337.5 \end{bmatrix}$$

$$= - \begin{bmatrix} 0.012 & 0.015 \\ 0.015 & 0.040 \end{bmatrix} \begin{bmatrix} -2025 \\ 900 \end{bmatrix}$$

(2x2) (2x1)

$$= \begin{bmatrix} -24.3 + 13.5 \\ -30.37 + 36 \end{bmatrix}$$

$$[x] = \begin{bmatrix} 10.8 \\ -5.63 \end{bmatrix}$$

10. Internal member force :-

$$[Q] = \begin{bmatrix} 3 & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \\ 0 & 0 & 0 \\ 0 & 0 & 0 \end{bmatrix} \begin{bmatrix} 50 \\ 75 \\ -75 \end{bmatrix} + \begin{bmatrix} -6 & 0 \\ 6 & -3 \\ -6 & 3 \\ 0 & -3 \\ 0 & 3 \\ 0 & 0 \end{bmatrix} \begin{bmatrix} 10.8 \\ -5.63 \end{bmatrix} + \begin{bmatrix} 0 \\ 0 \\ -75 \\ 75 \\ 0 \\ 0 \end{bmatrix}$$

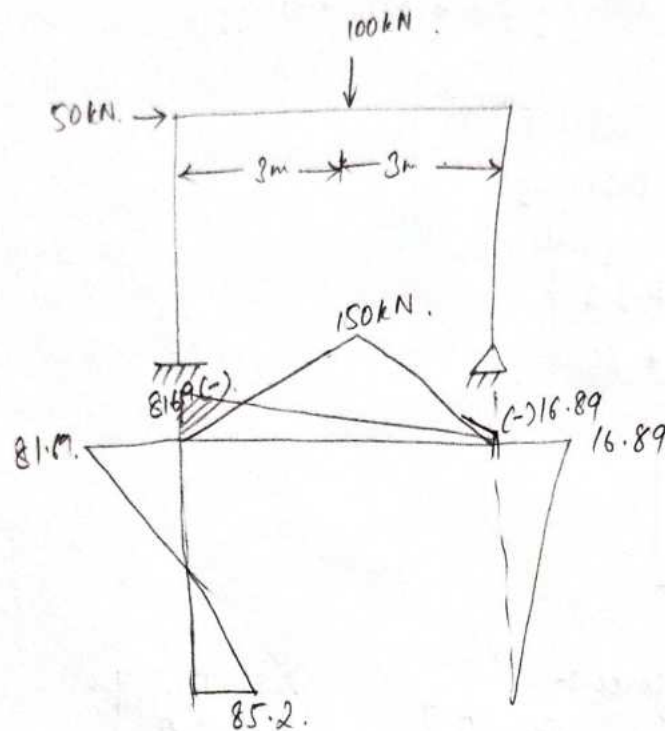
$$= \begin{bmatrix} 150 \\ 0 \\ 75 \\ -75 \\ 0 \\ 0 \end{bmatrix} + \begin{bmatrix} -64.8 \\ +64.8 + 16.89 \\ -64.8 - 16.89 \\ 16.89 \\ -16.89 \\ 0 \end{bmatrix} + \begin{bmatrix} 0 \\ 0 \\ -75 \\ 75 \\ 0 \\ 0 \end{bmatrix}$$

$$= \begin{bmatrix} 150 \\ 0 \\ 75 \\ -75 \\ 0 \\ 0 \end{bmatrix} + \begin{bmatrix} -64.8 \\ 81.69 \\ -81.69 \\ +16.89 \\ -16.89 \\ 0 \end{bmatrix} + \begin{bmatrix} 0 \\ 0 \\ -75 \\ 75 \\ 0 \\ 0 \end{bmatrix} = \begin{bmatrix} 85.2 \\ 81.69 \\ -81.69 \\ 16.89 \\ -16.89 \\ 0 \end{bmatrix}$$

Step 11:- Final Moment .

$$\begin{bmatrix} M_{AB} \\ M_{BA} \\ M_{BC} \\ M_{CB} \end{bmatrix} = \begin{bmatrix} 85.2 \\ 81.69 \\ -81.69 \\ 16.89 \\ 0 \end{bmatrix}$$

Step 12:- ^{M_{CD}}_{M_{DC}} Maximum Bending Moment .



$$\begin{aligned} \text{Max. B.M.} &= \frac{wl}{4} = \frac{100 \times 6}{4} \\ &= 150 \text{ kNm} . \end{aligned}$$

Date: 11/04/2021

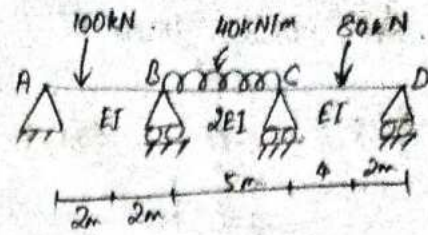
Q. Analyse the continuous beam as shown in fig. by flexibility matrix method:

Ans: Step 1: Static Indeterminacy.

$$D_s = R - K$$

$$= (5 - 3) \quad 4 - 2$$

$$= 2$$



(Vertical Loading Condition).

M_B and M_C are considered to be redundant.

Step 2: Fixed End Moments.

$$M_{FAB} = \frac{wab^2}{l^2} = 100 \times$$

$$\frac{wab^2}{l^2} / \frac{wab^2}{l^2}$$

$$\max B.M. = \frac{wab}{l}$$

$$M_{FAB} = -\frac{wl}{8} = -\frac{100 \times 4}{8} = -50 \text{ kNm}$$

$$M_{FBA} = \frac{wl}{8} = 50 \text{ kNm}$$

$$M_{FBC} = -\frac{wl^2}{12} = -\frac{40 \times 5^2}{12} = -83.3 \text{ kNm}$$

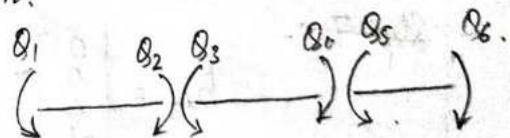
$$M_{FCB} = \frac{wl^2}{12} = 83.3 \text{ kNm}$$

$$M_{FCD} = -\frac{wab^2}{l^2} = \frac{80 \times 4 \times 2^2}{6^2} = -35.56 \text{ kNm}$$

$$M_{FDC} = \frac{wab^2}{l^2} = \frac{80 \times 4^2 \times 2}{6^2} = 71.1 \text{ kNm}$$

Step 3: Internal member force action.

$$Q = \begin{Bmatrix} -50 \\ 50 \\ -83.3 \\ 83.3 \\ -35.56 \\ 71.1 \end{Bmatrix}$$



Step 4: Equivalent joint moment.

$$R = \begin{Bmatrix} 50 \\ 33.3 \\ -47.8 \\ -71.1 \end{Bmatrix}$$

Step 5:- Force transformation matrix.

Apply unit force at every points.

$$\{b_{R1}\} = \begin{Bmatrix} 1 \\ 0 \\ 0 \\ 0 \\ 0 \end{Bmatrix} \quad \{b_{R2}\} = \begin{Bmatrix} 0 \\ 1 \\ 0 \\ 0 \\ 0 \end{Bmatrix}$$

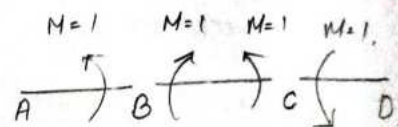
$$\{b_{R3}\} = \begin{Bmatrix} 0 \\ 0 \\ 1 \\ 0 \\ 0 \end{Bmatrix} \quad \{b_{R4}\} = \begin{Bmatrix} 0 \\ 0 \\ 0 \\ 1 \\ 0 \end{Bmatrix}$$

$$\{b_R\} = \begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 1 \\ 0 & 0 & 0 & 0 \end{bmatrix}$$



Step 6:- Moment transformation matrix (b_M).

$$b_{M1} = \begin{Bmatrix} 0 \\ -1 \\ 1 \\ 0 \\ 0 \end{Bmatrix}$$



Step 7:-

$$b_{M2} = \begin{Bmatrix} 0 \\ 0 \\ 0 \\ -1 \\ 1 \end{Bmatrix}$$

$$b_M = \begin{bmatrix} 0 & 0 \\ -1 & 0 \\ 1 & 0 \\ 0 & -1 \\ 0 & 1 \end{bmatrix}$$

Step 7:- Force Matrix.

$$F_{AB} = \frac{l}{6EI} \begin{bmatrix} 2 & -1 \\ -1 & 2 \end{bmatrix} = \frac{4}{6EI} \begin{bmatrix} 2 & -1 \\ -1 & 2 \end{bmatrix} = \frac{1}{EI} \begin{bmatrix} 1.34 & -0.67 \\ -0.67 & 1.34 \end{bmatrix}$$

$$F_{BC} = \frac{5}{6EI \times 2} \begin{bmatrix} 2 & -1 \\ -1 & 2 \end{bmatrix} = \begin{bmatrix} 1.66 & -0.83 \\ -0.83 & 1.66 \end{bmatrix} \frac{1}{2EI} = \begin{bmatrix} 0.83 & -0.42 \\ -0.42 & 0.83 \end{bmatrix} = \frac{1}{2EI} \begin{bmatrix} 2.76 & -0.69 \\ -0.69 & 2.76 \end{bmatrix} = \frac{1}{2.07EI}$$

$$F_{CD} = \frac{6}{6EI} \begin{bmatrix} 2 & -1 \\ -1 & 2 \end{bmatrix} = \frac{1}{EI} \begin{bmatrix} 2 & -1 \\ -1 & 2 \end{bmatrix}$$

$$F = \frac{1}{EI} \begin{bmatrix} 1.34 & -0.67 & 0 & 0 & 0 & 0 \\ -0.67 & 1.34 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0.83 & -0.42 & 0 & 0 \\ 0 & 0 & -0.42 & 0.83 & 0 & 0 \\ 0 & 0 & 0 & 0 & 2 & -1 \\ 0 & 0 & 0 & 0 & -1 & 2 \end{bmatrix}$$

Step 8:- Flexibility Matrix .

$$F_{xx} = [b_x]^T [F] [b_x] .$$

$$= \frac{1}{EI} \begin{bmatrix} 0 & -1 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & -1 & 1 & 0 \end{bmatrix} \begin{bmatrix} 1.34 & -0.67 & 0 & 0 & 0 & 0 \\ -0.67 & 1.34 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0.83 & -0.42 & 0 & 0 \\ 0 & 0 & -0.42 & 0.83 & 0 & 0 \\ 0 & 0 & 0 & 0 & 2 & -1 \\ 0 & 0 & 0 & 0 & -1 & 2 \end{bmatrix} \begin{bmatrix} 0 & 0 \\ -1 & 0 \\ 1 & 0 \\ 0 & -1 \\ 0 & 1 \\ 0 & 0 \end{bmatrix}$$

(6x6) (6x2)

$$= \frac{1}{EI} \begin{bmatrix} 0 & -1 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & -1 & 1 & 0 \end{bmatrix} \begin{bmatrix} 0.67 & 0 \\ -1.34 & 0 \\ 0.83 & 0.42 \\ -0.42 & -0.83 \\ 0 & 2 \\ 0 & -1 \end{bmatrix}$$

(2x6) (6x2)

$$= \frac{1}{EI} \begin{bmatrix} 1.34 + 0.83 & 0.42 \\ 0.42 & 0.83 + 2 \end{bmatrix}$$

$$[F_{xx}] = \frac{1}{EI} \begin{bmatrix} 2.17 & 0.42 \\ 0.42 & 2.83 \end{bmatrix} \quad (6.14 - 0.18)$$

Step 9:-

$$[A] = \frac{1}{|A|} (\text{adj } A) .$$

$$= \frac{EI}{5.96} \begin{bmatrix} 2.83 & -0.42 \\ -0.42 & 2.17 \end{bmatrix}$$

$$= EI \begin{bmatrix} 0.47 & -0.07 \\ -0.07 & 0.86 \end{bmatrix}$$

Step 9:- Internal member force.

$$\{S\} = \{b_x\}[R] + \{b_x\}\{x\} + FEM.$$

$$= \begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 \end{bmatrix} \begin{bmatrix} 50 \\ 33.3 \\ -47.8 \\ -71.1 \end{bmatrix} + \begin{bmatrix} 0 & 0 \\ -1 & 0 \\ 1 & 0 \\ 0 & -1 \\ 0 & 1 \\ 0 & 0 \end{bmatrix}$$

Step 9:- Redundant Force Matrix.

$$\{x\} = -[F_{xx}]^{-1} [F_{xr}][R].$$

$$[F_{xr}] = \{b_x\}^T [F] \{b_r\}$$

$$= \begin{bmatrix} 0 & -1 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & -1 & 1 & 0 \end{bmatrix} \frac{1}{EI} \begin{bmatrix} 1.34 & -0.67 & 0 & 0 & 0 & 0 \\ -0.67 & 1.34 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0.83 & -0.42 & 0 & 0 \\ 0 & 0 & -0.42 & 0.83 & 0 & 0 \\ 0 & 0 & 0 & 0 & 2 & -1 \\ 0 & 0 & 0 & 0 & -1 & 2 \end{bmatrix}$$

$$\begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 \end{bmatrix}$$

$$= \frac{1}{EI} \begin{bmatrix} 0 & -1 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & -1 & 1 & 0 \end{bmatrix} \quad (2 \times 6)$$

$$\begin{bmatrix} 1.34 & -0.67 & 0 & 0 \\ -0.67 & 1.34 & 0 & 0 \\ 0 & 0 & -0.42 & 0 \\ 0 & 0 & 0.83 & 0 \\ 0 & 0 & 0 & -1 \\ 0 & 0 & 0 & 2 \end{bmatrix} \quad (6 \times 4)$$

(6x4)

$$= \frac{1}{EI} \begin{bmatrix} -0.67 & -1.34 & -0.42 & 0 \\ 0 & 1 & -0.83 & -1 \end{bmatrix}$$

$$[x] = -[F_{xx}]^{-1} [F_{xR}] [R]$$

$$= -\frac{EI}{EI} \begin{bmatrix} 0.47 & -0.07 \\ -0.07 & 0.36 \end{bmatrix} \times \frac{1}{EI} \begin{bmatrix} 0.67 & -1.34 & -0.42 & 0 \\ 0 & 0 & -0.83 & -1 \end{bmatrix} \begin{bmatrix} 50 \\ 33.3 \\ -47.8 \\ -71.1 \end{bmatrix} \begin{matrix} (4 \times 1) \\ (2 \times 4) \end{matrix}$$

$$= \begin{bmatrix} 0.47 & -0.07 \\ -0.07 & 0.36 \end{bmatrix} \begin{bmatrix} 33.5 - 44.62 + 20.07 + 0 \\ 39.67 + 71.1 \end{bmatrix}$$

$$= \begin{bmatrix} 0.47 & -0.07 \\ -0.07 & 0.36 \end{bmatrix} \begin{bmatrix} 8.95 \\ 110.77 \end{bmatrix}$$

$$= \begin{bmatrix} 4.206 - 7.753 \\ -0.6265 + 39.877 \end{bmatrix}$$

$$16.91$$

$$-13.57$$

$$= \begin{bmatrix} 3.55 \\ -39.25 \end{bmatrix}$$

Step 10 :- Internal member force

$$[Q] = [b_R] [R] + [b_x] [x] + FEM$$

$$= \begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 1 \end{bmatrix} \begin{bmatrix} 50 \\ 33.3 \\ -47.8 \\ -71.1 \end{bmatrix} + \begin{bmatrix} 0 & 0 \\ -1 & 0 \\ 1 & 0 \\ 0 & -1 \\ 0 & 1 \\ 0 & 1 \\ 0 & 0 \end{bmatrix} \begin{bmatrix} +3.55 \\ -39.25 \end{bmatrix} + \begin{bmatrix} -50 \\ 50 \\ -83.3 \\ 83.3 \\ -35.56 \\ 71.1 \end{bmatrix}$$

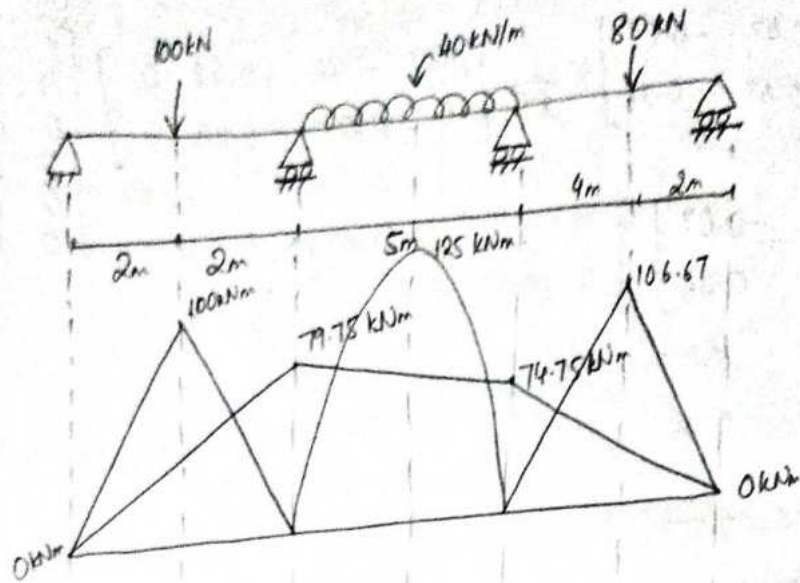
$$= \begin{bmatrix} 50 \\ 33.3 \\ 0 \\ -47.8 \\ 0 \\ -71.1 \end{bmatrix} + \begin{bmatrix} 0 \\ -3.55 \\ 3.55 \\ -39.25 \\ -39.25 \\ 0 \end{bmatrix} + \begin{bmatrix} -50 \\ 50 \\ -83.3 \\ 83.3 \\ -35.56 \\ 71.1 \end{bmatrix} = \begin{bmatrix} 0 \\ 79.75 \\ -79.75 \\ 74.75 \\ -74.81 \\ 0 \end{bmatrix}$$

Step 11 :- Final Moment

$$\begin{bmatrix} M_{AB} \\ M_{BA} \\ M_{BC} \\ M_{CB} \\ M_{CD} \\ M_{DC} \end{bmatrix} = \begin{bmatrix} 0 \\ 86.88 \\ -86.85 \\ 3.75 \\ 3.69 \\ 0 \end{bmatrix} \begin{bmatrix} 0 \\ 79.75 \\ -79.75 \\ 74.75 \\ -74.81 \\ 0 \end{bmatrix}$$

Date: 13/04/2022

Flexibility Method :-



$$\begin{aligned}
 M_{AB} &= \frac{wl}{4} = \frac{100 \times 4}{4} = 100 \text{ kNm} \\
 M_{BC} &= \frac{wl^2}{8} = \frac{40 \times 5^2}{8} = 125 \text{ kNm} \\
 M_{CD} &= \frac{wab}{1} = \frac{80 \times 4 \times 2}{6} = 106.67 \text{ kNm}
 \end{aligned}$$

Bending Moment