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COIMBATORE-641 035, TAMIL NADU



DEPARTMENT OF AEROSPACE ENGINEERING

To illustrate how we can combine spring and bar elements in one structure, we now solve the two-bar truss supported by a spring shown in Figure .1. Both bars have $E= 210 \text{ GPa}$ and $A= 5 \times 10^4 \text{ m}^2$. Bar one has a length of 5 m and bar two a length of 10 m. The spring stiffness is $k = 2000 \text{ kN/m}$.

Solution:

Stiffness matrix

$$k = \frac{AE}{L} \begin{bmatrix} l^2 & lm & -l^2 & -lm \\ lm & m^2 & -lm & -m^2 \\ -l^2 & -lm & l^2 & lm \\ -lm & -m^2 & lm & m^2 \end{bmatrix}$$

Element ① $\theta = 135^\circ$

$$l = \cos 135 = -0.70710678 \times 10^3$$

$$l^2 = 0.5$$

$$m = \sin 135 = +0.70710678 \times 10^3$$

$$m^2 = 0.5 \quad [lm = -0.5]$$

Element ② $\theta = 45^\circ$

$$l = \cos 45 = 0.70710678 \times 10^3$$

$$l^2 = 0.5$$

$$m = \sin 45 = 0.70710678 \times 10^3$$

$$m^2 = 0.5 \quad [lm = 0.5]$$

Two-bar truss with spring support

Stiffness matrix for Element ①

$$K^{(1)} = \frac{(5 \times 10^4) \times (210 \times 10^9)}{5} \begin{bmatrix} 0.5 & -0.5 & -0.5 & 0.5 \\ -0.5 & 0.5 & 0.5 & -0.5 \\ -0.5 & 0.5 & 0.5 & -0.5 \\ 0.5 & -0.5 & -0.5 & 0.5 \end{bmatrix}$$

$$= 105 \times 10^5 \begin{bmatrix} 1 & -1 & -1 & 1 \\ -1 & 1 & 1 & -1 \\ -1 & 1 & 1 & -1 \\ 1 & -1 & -1 & 1 \end{bmatrix}$$

Diagram Description: A truss structure with nodes 1, 2, 3, 4. Node 1 is at the top right, node 2 is at the top left, node 3 is at the bottom left, and node 4 is at the bottom right. A horizontal bar of length 10 m connects nodes 2 and 1. A diagonal bar of length 5 m connects nodes 2 and 1. A vertical spring of stiffness $k = 2000 \text{ kN/m}$ connects nodes 1 and 4. A 50 kN load is applied at node 1 at an angle of 135° to the horizontal. Displacements are labeled as $u_1, u_2, u_3, u_4, u_5, u_6, u_7, u_8$.

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Element ②, $\theta = 180^\circ$

$$l^2 = 1 \quad lm = 0 \quad m^2 = 0$$

Stiffness matrix for Element ②
nodes [1-3]

$$L = 10 \text{ m}$$

$$K^{(2)} = \frac{(5 \times 10^{-4}) \times (210 \times 10^9)}{10} \begin{bmatrix} 1 & 0 & -1 & 0 \\ 0 & 0 & 0 & 0 \\ -1 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 \end{bmatrix}$$
$$= 105 \times 10^5 \begin{bmatrix} 1 & 0 & -1 & 0 \\ 0 & 0 & 0 & 0 \\ -1 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 \end{bmatrix}$$

Element ③, $\theta = 270^\circ$

$$l^2 = 0 \quad lm = 0 \quad m^2 = 1$$

Stiffness matrix for Element ③
nodes [1-4]

$$K^{(3)} = 20 \times 10^5 \begin{bmatrix} 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & -1 \\ 0 & 0 & 0 & 0 \\ 0 & -1 & 0 & 1 \end{bmatrix}$$

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General Finite Element equation: $[K] \{u\} = \{F\}$

1	2	3	4	5	6	7	8
105	-105	-105	105	-105	0	0	0
+105	-105	105	-105	0	0	0	0
-105	+105	105	-105	0	0	0	-20
105	-105	-105	105	0	0	0	0
-105	0	0	0	+105	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	-20	0	0	0	0	0	0

u_1	u_2	u_3	u_4	u_5	u_6	u_7	u_8
1	2	3	4	5	6	7	8
$F_1(0)$	$F_2(-50)$	$F_3(6)$	$F_4(6)$	$F_5(6)$	$F_6(6)$	$F_7(0)$	$F_8(0)$

Boundary conditions are

$u_3 = u_4 = u_5 = u_6 = u_7 = u_8 = 0$
The final matrix

$$105 \begin{bmatrix} 210 & -105 \\ -105 & 125 \end{bmatrix} \begin{bmatrix} u_1 \\ u_2 \end{bmatrix} = \begin{bmatrix} 0 \\ -50 \times 10^3 \end{bmatrix}$$

$$210 \times 10^5 u_1 - 105 \times 10^5 u_2 = 0 \quad \text{--- (1)}$$

$$-105 \times 10^5 u_1 + 125 \times 10^5 u_2 = -50 \times 10^3 \quad \text{--- (2)}$$

$$u_1 = -3.448 \times 10^{-3}$$

$$u_2 = -6.896 \times 10^{-3}$$

Stress: Stress of element (1)

$$\sigma_{11} = \frac{E}{L_1} [-l_1, -m_1, l_1, m_1] \begin{bmatrix} u_1 \\ u_2 \\ u_3 \\ u_4 \end{bmatrix}$$

$$\sigma_{11} = \frac{210 \times 10^9}{5} [0.707, -0.707, -0.707, 0.707] \begin{bmatrix} -3.448 \times 10^{-3} \\ -6.896 \times 10^{-3} \\ 0 \\ 0 \end{bmatrix}$$

$$= 102.4 \text{ MPa [T]}$$

σ_{12}

$$= \frac{E}{L_2} [-l_2, -m_2, l_2, m_2] \begin{bmatrix} u_1 \\ u_2 \\ u_5 \\ u_6 \end{bmatrix}$$

$$= \frac{210 \times 10^9}{10} [1, 0, -1, 0] \begin{bmatrix} -3.448 \times 10^{-3} \\ -6.896 \times 10^{-3} \\ 0 \\ 0 \end{bmatrix}$$

$$= -72.2 \text{ MPa [C]}$$

Note: can show equilibrium at node 1

$$F_s = [2000 \text{ kN/m}] \times [6.896 \times 10^{-3} \text{ m}] = 13.792 \text{ kN}$$

$$f_{1-3} = 35.6 \text{ kN} \quad \sum F_y = 0$$

$$f_{1-2} = 51.2 \text{ kN} \quad -50 + 13.79 + 36.198 = 0$$

σ_{23} PA

$$f_{1-2} = 102.4 \times 5 \times 10^{-4} = 51.2 \text{ kN}$$

$$f_{1-3} = -72.4 \times 5 \times 10^{-4} = -36.2 \text{ kN}$$